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## Preliminary Traffic Report <br> West of Missoula - NW (Mullan Road)

STPS 263-1(28)6
UPN 6141000

Missoula, MT
July 2018

## Table of Contents

Introduction ..... 1
Existing Conditions ..... 2
Study Location ..... 2
Previous Studies ..... 3
Traffic Data ..... 3
Crash History ..... 4
Intersection Operations ..... 5
Proposed Conditions ..... 6
Roadway Improvements ..... 6
Crash Reduction. ..... 7
Intersection Operations ..... 7
Conclusions and Recommendations ..... 8

## Introduction

Mullan Road is a north-south roadway connecting Interstate 90 at Frenchtown, Montana to Broadway Street in Missoula, Montana. It is functionally classified as a major collector between Frenchtown and Missoula and as a minor arterial once within Missoula city limits. The existing roadway follows the same path as a historic wagon road of the same name. In fact, it was the first wagon road to cross the Rocky Mountains and access the Pacific Northwest inland. Mullan Road provides access to largely rural residences, as well as Council Grove State Park along the Clark Fork River. It also provided access to an abandoned large pulp mill near the intersection of Pulp Mill Road. The mill closed on December 14 ${ }^{\text {th }}, 2009$, and was reported to be in the process of cleanup in April 2017.

This Montana Department of Transportation (MDT) project will fully reconstruct the road surface due to its deteriorating existing condition. The reconstruction will include safety enhancements such as revised curvature, widened shoulders, improved clear zone, updated signing and striping, and added rumble strips. Project limits extend between reference point (RP) 5.5, west of Deschamps Lane, and RP 10.6, north of the intersection with Pulp Mill Road. Figure 1 shows the project vicinity.


Figure 1. Project Vicinity
At the north end of the project is the Mullan Road and Pulp Mill Road intersection, a three-way stop controlled intersection with stop control on both Mullan Road approaches and the Pulp Mill Road approach. There is an uncontrolled approach to the intersection from the mill site. At the south end of the project is the intersection of Mullan Road and Deschamps Lane, a three-legged stop-controlled intersection with stop control on the Deschamps Lane approach. The
intersection currently has Deschamps Lane at an extreme skew, and this project will realign it to be perpendicular to the mainline and improve sight distance at the public approach.

The purpose of this Preliminary Traffic Report is to assess the safety and traffic operations of the existing Mullan Road and intersections as well as proposed improvements. The safety assessment will compare crash history with proposed countermeasures in order to identify whether additional improvements are needed. The operational assessment will analyze intersection performance using future volume projections to determine adequate proposed lane configurations at the two intersections previously mentioned.

## Existing Conditions

## Study Location

Mullan Road, also referred to as Secondary Route 263 (S-263), is functionally classified as a major collector within the project area, with speed limits of 55 miles per hour (mph) from RP 5.5 to $9.9,45 \mathrm{mph}$ from RP 9.9 to 10.0, and 35 mph from RP 10.0 to 10.6. It has one 12 ' travel lane in each direction and a paved shoulder width of 1 ' or less. It is an undivided roadway with white longitudinal pavement markings on both edges and a double yellow center line that changes with skip markings to allow passing in both directions in designated passing zones.

Just south of the mill site is a railroad crossing for a spur line that accessed the mill. This project will not make changes to the crossing. The crossing currently has train-activated flashing lights but no gate. This crossing should be considered for gates if rail traffic resumes.

There are several active transportation features along the Mullan Road segment to note. Approximately 180 -feet south of the intersection with Pulp Mill Road is a continuously flashing marked pedestrian crossing across Mullan Road. It was installed to allow employees who parked on the east side of Mullan Road to cross the highway to the mill. There are stop bars on Mullan Road in each direction before the marked crosswalk. A short segment of shared use path is on the north side of the roadway between RP 6.45 and RP 6.69. No other dedicated pedestrian/bicycle/ADA features exist in the project area.

Pulp Mill Road, also referred to as S-474, is functionally classified as a major collector with a posted speed limit of 35 mph near the project area. It has one 12 ' travel lane in each direction and a 1' paved shoulder. It is an undivided roadway with white longitudinal pavement markings on both edges and a double yellow center line that changes with skip markings to allow passing in both directions in designated passing zones.

Deschamps Lane, also referred to as County Rd 13, is functionally classified as a local road with a speed limit of 30 mph . It has one $10^{\prime}$ gravel travel lane in each direction and 2' to 3 ' dirt shoulder on either side. It is an undivided roadway with no pavement markings.

The Mullan Road and Pulp Mill Road intersection is stop controlled on the Pulp Mill Road approach. It lies just to the east of the former pulp mill, which operated from 1956 to 2009. Both the northbound and southbound Mullan Road approaches have only one approach lane and
one exit lane, and the same is true for the Pulp Mill Road approach. There are stop signs located at both Mullan Road approaches and the one Pulp Mill Road approach, and the speed limit is 35 mph on Mullan and Pulp Mill Roads near the intersection. The eastbound approach of the intersection served as the main entrance for the now closed pulp mill. The entrance is approximately 110 ' wide and has no stop sign or other control or positive guidance to move vehicles to the intersection.

The Mullan Road and Deschamps Lane intersection is stop controlled and has three approaches. The southbound Deschamps Lane approach is the only approach with a stop sign, with both Mullan approaches being free movements. Deschamps Lane intersects Mullan Road at a very acute angle with a channelized gravel/dirt right turn lane on Deschamps Lane that allows for right turns onto Mullan Road, and left turns from Mullan Road onto Deschamps Lane. Mullan Road has a speed limit of 55 mph through the intersection.

## Previous Studies

MDT approved the project's preliminary field review report in October 2017 based on a field review that occurred on August $15^{\text {th }}, 2016$. The report included existing physical characteristics and major design features, right-of-way details, existing curve analysis, crash analysis, traffic data, and work zone safety and mobility overview. Project improvements will be based on the deficiencies that the field review report identified, which include substandard curves, minimal shoulder, and steep side slopes.

## Traffic Data

AADT projections for opening year (2021) and design year (2041) on Mullan Road within the study area were provided by MDT for this analysis. Further data such as average or $85^{\text {th }}$ percentile speeds, pedestrian or bicycle volumes, and origin-destination information was not available so analysis of speeds, origin-destination distribution or pedestrian facilities were not conducted. Design traffic data used for this project is presented in Figure 2 and all provided traffic data is attached in Appendix A.


Figure 2. Design Traffic Data

Turning movement counts were taken by All Traffic Data on Tuesday, January $16^{\text {th }}, 2018$ at the Mullan Road and Pulp Mill Road and Mullan Road and Deschamps Lane intersections. The determined AM and PM peak hours are shown in Table 1 below. Turning movement count data also is presented in Appendix A.

Table 1. Peak hour times

| Intersection | AM Peak <br> Hour | PM Peak <br> Hour |
| :--- | :--- | :--- |
| Mullan Rd \& | 7:30 am to | $4: 30 \mathrm{pm}$ to |
| Pulp Mill Rd | $8: 30 \mathrm{am}$ | $5: 30 \mathrm{pm}$ |
| Mullan Rd \& | 7:00 am to | $4: 45 \mathrm{pm}$ to |
| Deschamps Ln | 8:00 am | $5: 45 \mathrm{pm}$ |

## Crash History

Crash data within the project area was provided by MDT in October 2017 and included all data within the Montana Highway Patrol records for the ten-year period of July 1, 2007 to June 30, 2017 for Mullan Road from RP 3.5 to 10.7.

Crashes
There was a total of 85 non-junction related crashes along Mullan Road within the project area during the study period. Two of these crashes were fatal injury crashes, 17 incapacitating injury crashes, 7 possible injury crashes, and 59 were property damage only (PDO). One fatal crash was a rear-end crash at RP 5.9 when a vehicle tried to avoid an animal in the roadway. It appears from the data that the first vehicle stopped suddenly, and two following vehicles both rear ended, being unable to stop. It occurred during the day under dry conditions on a straight and level segment of road. The second fatal crash involved an impaired driver striking a mailbox at night at RP 7.1, which lies on a horizontal curve. Roughly two-thirds of non-junction related crashes occurred at night, and 18 involved alcohol or drug impaired drivers. Most crashes were either fixed object crashes or rollover crashes. Fixed object crashes occur when a vehicle leaves the travel lane and hits an object such as a fence, embankment, or mailbox. Rollover crashes occur when a vehicle departs the roadway and rolls over. There were 42 crashes with a fixed object, making up about half of the non-junction crashes on Mullan Road while rollover crashes accounted for 23 of the crashes, or little over $25 \%$.

In addition to the non-junction related crashes, four crashes occurred at the intersection of Mullan Road and Pulp Mill Road during the study period. Two were non-incapacitating injury crashes and the other two were PDO. Three crashes occurred at night, and one involved an alcohol or drug impaired driver. Three of the crashes were rear-end and the other a fixed object crash with a concrete barrier. Two of the rear end crashes occurred on the southbound approach of Mullan Road. The third rear-end crash was located on the westbound approach.

Three crashes occurred at the intersection of Mullan Road and Deschamps Lane during the study period. One crash was a fatal injury crash, and the other two property damage only
crashes. The fatal injury crash was a single-vehicle rollover crash involving an impaired driver. It was classified as a rollover crash even though the first harmful event was striking a pole or support. It was one of two of the crashes at the intersection that occurred at night. Of the three crashes here, one was a wild animal crash, another a rollover crash, and the other a fixed object crash.

The MDT Safety Section has provided information on the Level of Service of Safety (LOSS) for this roadway. Based on the same time parameter (July 1, 2007 through June 30, 2017), E. Mullan Road (CO31070E from RP 3.5 to RP 10.7 is operating at a LOSS IV rating for total crashes and at a LOSS III rating for fatal injury crashes. A LOSS IV rating is indicative of a high potential for crash reduction. Similarly, a LOSS III rating represents a moderate to high potential for crash reduction.

Using MDT pattern recognition tools, this section of roadway has both an observed total fixed object and off road left crash pattern. An observed crash pattern is identified as having a minimum number of 5 crashes and a $95 \%$ cumulative probability.

## Intersection Operations

The Highway Capacity Manual 2010 is the standard for determining roadway operational performance and has long used the concept of level of service (LOS) to correlate numerical traffic operational data to subjective descriptions of traffic performance at intersections. LOS ranges from " $A$ " (best) to " $F$ " (worst). Vehicle LOS at intersections is a measure of average vehicle delay. To identify intersection capacity deficiencies and improvement needs, HDR applied and LOS threshold of LOS C for the overall intersection operation. This is consistent with the threshold set for other traffic studies conducted in the Missoula Metropolitan Area.

Synchro 9 software was used to model and analyze the existing capacity conditions of the two study intersections on Mullan Road at Pulp Mill Road and Deschamps Lane. Turning movement counts provided by All Traffic Data were inputted into Synchro, as well as the truck percentage value of $7.7 \%$ that was provided by MDT.

During the a.m. peak hour, both intersections of Mullan Road and Pulp Mill Road and Mullan Road Deschamps Lane are estimated to operate at LOS A. All movements at both intersections are also estimated to operate at LOS A.

During the p.m. peak hour, both intersections of Mullan Road and Pulp Mill Road and Mullan Road Deschamps Lane are estimated to operate at LOS A. The southbound left movement on Deschamps Lane is estimated to operate at LOS B, and all other movements are estimated to operate at LOS A. While HCM 2010 methodology does not account for intersection skew, there were no borderline results that would become unacceptable if the skew were to have a slight negative impact.

No movement for either intersection in either peak hour exceeded a volume-to-capacity (V/C) ratio of 0.2.

## Proposed Conditions

## Roadway Improvements

Roadway improvements are determined by roadway facility and documented in the preliminary field report. The project will design a reconstruction of Mullan Road that will include 6' paved shoulders outside the 12 ' wide travel lanes. Beyond designing the roadway to current design standards, additional safety features that will be included as part of the project are listed below:

- Improve sight distance and intersection skew angle at the Deschamps Lane intersection
- Add centerline rumble strips

Since the pulp mill is no longer in operation, lower traffic volumes have prompted discussion on whether to remove the Mullan Road stop signs at Pulp Mill Road. During the PM peak hour, Mullan Road has 145 vehicles per hour (vph), and Pulp Mill Road has 92 vph. In the volume projections for 2041, this increases to 207 vph on Mullan Road and 132 vph on Pulp Mill Road. The Manual on Uniform Traffic Control Devices (MUTCD) Section 2B. 07 provides guidance on multi-way stop applications. Where the $85^{\text {th }}$-percentile speed is 40 mph or less, major street traffic should be at least 300 vph for any eight hours of an average day, and minor street traffic should be at least 200 units per hour (including vehicles, bicycles, and pedestrians) during those eight hours. Also, the minor street delay using two-way stop control should be at least 30 seconds per vehicle to warrant multi-way stop control. If the $85^{\text {th }}$-percentile speed on the major street exceeds 40 mph , these minimum values can be reduced to 70 percent of the original values. All of these requirements should be met to warrant multi-way stop control. Even at 70 percent, current traffic at the Mullan Road and Pulp Mill Road intersection does not meet MUTCD Section 2B. 07 criteria for any hour during the day. The 2041 peak hour traffic projections are close to the multi-way stop control criteria; however, even in this case they would not meet the criteria for eight hours in a day.

MUTCD Section 2B. 07 also provides the following options to justify multi-way stop applications:

- Five or more crashes were reported at the intersection within a twelve-month period and could have been corrected by multi-way stop control
- There is a need to control left turn conflicts or vehicle/pedestrian conflicts
- There is inadequate sight distance from the minor street
- Multi-way stop control would benefit the operational characteristics of two intersecting residential streets of similar nature.

The Mullan Road and Pulp Mill Road intersection meets none of these criteria. Since there is no justification for a multi-way stop controlled intersection at Mullan Road and Pulp Mill Road, the stop signs on Mullan Road can be removed at MDT's discretion. If the old mill site were to be reopened or repurposed, intersection control would need to be revisited before adding more traffic to the intersection.

Without stop control on Mullan Road at Pulp Mill Road, a southbound left turn lane may provide a safety benefit. This is discussed more below.

## Crash Reduction

Crash modification factors (CMFs) from the HSM provide a sense for the effectiveness of various crash countermeasures. A CMF can be multiplied by the number of existing crashes to help predict the number of future crashes. Where the HSM does not provide adequate data for improvements proposed for the project, relevant studies were found on cmfclearinghouse.org and referenced with their CMF ID number. This website uses a 5 -star rating system to indicate the quality of a study's results based on study design, sample size, standard error, controls for potential bias, and data source diversity.

From a qualitative standpoint, geometric improvements to the roadway are expected to yield the most significant safety benefits. Section 13.6.2 of the HSM specifically addresses countermeasures for rural, two-lane roads, such as Mullan Road. Three countermeasures it identified are:

- Increase horizontal curve radius and length, and providing spiral transitions
- Improve superelevation of horizontal curves
- Reduce vertical grades

These modifications will help reduce the number of roadway departure crashes along a rural, two-lane roadway, the most common crash type of all three crash clusters identified along Mullan Road.

The angle between Mullan Road and Deschamps Lane at the intersection is approximately 20 degrees. In HSM Section 10.7.2, a 90-degree intersection has a CMF of 1.00, while a 20-degree intersection has a CMF of 1.32. Realigning Deschamps Lane to a 90-degree intersection would reduce the CMF by 0.32 divided by 1.32 , or $24 \%$.

Table 9-23 in the 2011 Policy on Geometric Design of Highways and Streets, developed by the American Association of State Highway and Transportation Officials (AASHTO), provides guidelines on when to install left turn lanes. The projected 2041 volume at Mullan Road and Pulp Mill Road is 93 vph southbound, approximately $30 \%$ of which turn left, and 111 vph northbound opposing the left turn. Even if Mullan Road had an operating speed of 60 mph , it would need eight times as many left turning vehicles to justify a left turn lane. If the stop signs are removed from both Mullan Road approaches to the intersection, a left turn lane could reduce the risk of rear-end crashes by allowing left-turning vehicles to slow down out of the way of through movement traffic.

## Intersection Operations

Synchro 9 software was again used to model and analyze the operations of the two study intersections on Mullan Road at Pulp Mill Road and Deschamps Lane, this time with forecast volumes for both the letting year 2021 and the design year 2041. Turning movement counts were increased in Synchro by a growth factor for each year calculated using the forecast AADT data provided by MDT. The growth factor was equal to the forecast year AADT divided by the present year AADT. The intersections were modeled with the existing number of lanes and control types. Model alternatives were also created with added left turn lanes on the southbound
approaches of Mullan Road at intersections, as well as the Mullan Road and Pulp Mill Road intersection modeled with a stop sign only on the westbound Pulp Mill Road approach. The results described below are detailed in Appendix B. While HCM 2010 methodology does not account for intersection skew, there were no borderline results at Deschamps Lane that would become unacceptable if the skew were to have a slight negative impact.

The Mullan Road and Pulp Mill Road intersection is projected to operate at LOS A with an average intersection delay of 8 seconds per vehicle in 2021 and 2041 in both the a.m. and p.m. peak hours. If the stop signs on Mullan Road are removed and a southbound left turn lane is added, the average intersection delay is expected to drop to 4 to 5 seconds per vehicle. For two-way stop controlled intersections, the worst movement defines the LOS since many vehicles do not stop. The worst movement in 2021 and 2041 in the a.m. and p.m. peak hours is the westbound left turn movement and is estimated to perform at LOS B. Thus, removing the Mullan Road stop signs is estimated to provide a net benefit of 3 to 4 seconds delay reduction per vehicle without much detriment to Pulp Mill Road.

The Mullan Road and Deschamps Lane intersection is projected to operate with an average intersection delay of 1 to 2 seconds per vehicle in 2021 and 2041 in both the a.m. and p.m. peak hours. The worst movement on Deschamps Lane is the westbound left turn movement and is estimated to operate at LOS B. An added southbound left turn lane is not estimated to change the average intersection delay or LOS on the worst movement.

No movement at either intersection for any peak hour exceeded a volume-to-capacity (V/C) ratio of 0.2 .

## Conclusions and Recommendations

Estimated traffic operations for the intersections in the study area perform at LOS B or better for existing and forecast conditions. Therefore, the main benefit of this project, besides mitigating roadway deterioration, is the potential safety improvements for the traveling public.

Based on the analysis in this report, the following recommendations are expected to improve safety in the project area:

- Improve horizontal curvature, superelevation, and vertical curvature to meet current standards.
- Reduce vertical grades where possible.
- Widen shoulders.
- Improve side slopes and remove roadside obstacles to meet current clear zone standards.
- Install guardrail where adequate clear zone is not feasible.
- Install centerline and shoulder rumble strips.
- Consider removing stop signs on Mullan Road at Pulp Mill Road
- Realign Deschamps Lane to intersect Mullan Road at a 90-degree angle.


## Appendix A: Traffic Data

# Montana Department of Transportation Helena, Montana 59620 

## Memorandum

| To: | Fred Bente <br> Helena Consultant Design <br> Consultant Project Supervisor |
| :--- | :--- |
| From: | Becky Duke, Supervisor <br> Traffic Data Collection \& Analysis Section |
| Date: | August 10, 2017 |
| Subject: | STPS 263-1 (28)6 <br> West of Missoula - NW <br> Control No. 6141 |

Attached is the traffic information requested in an email dated August 7, 2017. There are no major traffic breaks within the project. Please note that the equivalency factors used to calculate ESAL values are determined using information from our weigh-in-motion sites and reflect a five-year average.

If you have any questions or need further assistance, please contact me at 6122 .

CC: Pavement Analysis and Research - Helena
Project File

## RAIL TRANSIT AND PLANNING DIVISION TRAFFIC DATA COLLECTION SECTION Worksheet for Engineering and Planning Purposes



2016

| AADT | $=$1,640 <br> BUS$=\frac{1.5 \%}{} \quad 25$ |
| ---: | :--- |
| COM | $=\frac{7.7 \%}{1.5 \%}$ |
| AGR | $=\frac{126}{10.40 \%}$ |
| K Factor | $=1$ |

* Distribution: 2016 Vehicle Class count (Site ID: 32-3A-036)
* AADT \& Growth Rate: 2016 TYC

(303) 216-2439 www.alltrafficdata.net

Location: 1 MULLAN RD \& PULP MILL RD AM
Date and Start Time: Tuesday, January 16, 2018
Peak Hour: 07:30 AM - 08:30 AM
Peak 15-Minutes: 08:00 AM - 08:15 AM

## Peak Hour - All Vehicles



## Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

## Traffic Counts

| Interval | PULP MILL RD <br> Eastbound |  |  |  | PULP MILL RD <br> Westbound |  |  |  | MULLAN RD Northbound |  |  |  | MULLAN RD Southbound |  |  |  | Total | Rolling Hour | Pedestrain Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left | Thru R | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 7:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 4 | 5 | 0 | 9 | 4 | 0 | 23 | 150 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 5 | 8 | 0 | 8 | 8 | 0 | 32 | 183 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 1 | 0 | 0 | 0 | 2 | 0 | 6 | 0 | 0 | 9 | 12 | 0 | 6 | 11 | 0 | 47 | 192 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 9 | 0 | 0 | 9 | 9 | 0 | 9 | 8 | 0 | 48 | 180 | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 13 | 0 | 0 | 13 | 10 | 0 | 8 | 9 | 0 | 56 | 149 | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 2 | 0 | 0 | 8 | 3 | 0 | 14 | 11 | 1 | 41 |  | 0 | 0 | 0 | 0 |
| 8:30 AM | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 5 | 0 | 0 | 7 | 5 | 0 | 4 | 10 | 0 | 35 |  | 0 | 0 | 0 | 0 |
| 8:45 AM | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 1 | 0 | 0 | 2 | 6 | 0 | 3 | 3 | 0 | 17 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 1 | 1 | 0 | 0 | 17 | 0 | 38 | 0 | 1 | 57 | 58 | 0 | 61 | 64 | 1 | 299 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 0 | 1 | 1 | 0 | 0 | 10 | 0 | 30 | 0 | 0 | 39 | 34 | 0 | 37 | 39 |  | 1192 |  | 0 | 0 | 0 | 0 |

All Traffic Data
Services Inc.
(303) 216-2439 www.alltrafficdata.net

Location: 2 MULLAN RD \& DESCHAMPS LN AM Date and Start Time: Tuesday, January 16, 2018
Peak Hour: 07:00 AM - 08:00 AM
Peak 15-Minutes: 07:30 AM - 07:45 AM

Peak Hour - All Vehicles


## Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

## Traffic Counts


(303) 216-2439 www.alltrafficdata.net

Location: 1 MULLAN RD \& PULP MILL RD PM
Date and Start Time: Tuesday, January 16, 2018
Peak Hour: 04:30 PM - 05:30 PM
Peak 15-Minutes: 05:00 PM - 05:15 PM

## Peak Hour - All Vehicles



## Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

## Traffic Counts

| Interval | PULP MILL RD <br> Eastbound |  |  |  | PULP MILL RD <br> Westbound |  |  |  | MULLAN RD <br> Northbound |  |  |  | MULLAN RD <br> Southbound |  |  |  | Total | Rolling Hour | Pedestrain Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left | Thru |  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 4:00 PM | 0 | 3 | 0 | 0 | 0 | 13 | 0 | 7 | 0 | 0 | 16 | 2 | 0 | 3 | 11 | 1 | 56 | 219 | 0 | 0 | 0 | 0 |
| 4:15 PM | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 8 | 0 | 0 | 11 | 9 | 0 | 3 | 12 | 0 | 54 | 231 | 0 | 0 | 0 | 0 |
| 4:30 PM | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 16 | 0 | 0 | 14 | 1 | 0 | 5 | 10 | 1 | 58 | 237 | 0 | 0 | 0 | 0 |
| 4:45 PM | 0 | 0 | 0 | 0 | 0 | 4 | 1 | 13 | 0 | 1 | 15 | 6 | 0 | 3 | 8 | 0 | 51 | 234 | 0 | 0 | 0 | 0 |
| 5:00 PM | 0 | 0 | 0 | 0 | 0 | 16 | 0 | 11 | 0 | 0 | 17 | 3 | 0 | 7 | 14 | 0 | 68 | 226 | 0 | 0 | 0 | 0 |
| 5:15 PM | 0 | 1 | 1 | 0 | 0 | 6 | 0 | 12 | 0 | 0 | 20 | 2 | 0 | 5 | 13 | 0 | 60 |  | 0 | 0 | 0 | 0 |
| 5:30 PM | 0 | 0 | 0 | 0 | 0 | 9 | 0 | 8 | 0 | 0 | 17 | 2 | 0 | 4 | 15 | 0 | 55 |  | 0 | 0 | 0 | 0 |
| 5:45 PM | 0 | 1 | 0 | 0 | 0 | 5 | 0 | 7 | 0 | 0 | 10 | 8 | 0 | 1 | 11 | 0 | 43 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 5 | 1 | 0 | 0 | 75 | 1 | 82 | 0 | 1 | 120 | 33 | 0 | 31 | 94 | 2 | 445 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 0 | 1 | 1 | 0 | 0 | 37 | 1 | 52 | 0 | 1 | 66 | 12 | 0 | 20 | 45 | 1 | 1237 |  | 0 | 0 | 0 | 0 |

(303) 216-2439
www.alltrafficdata.net
Location: 2 MULLAN RD \& DESCHAMPS LN PM
Date and Start Time: Tuesday, January 16, 2018
Peak Hour: 04:45 PM - 05:45 PM
Peak 15-Minutes: 05:00 PM - 05:15 PM

## Peak Hour - All Vehicles



## Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

## Traffic Counts



## Appendix B: Traffic Operational Analysis

| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh $\quad 7.5$ |  |
| Intersection LOS | A |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | ¢ |  |  | ¢ |  |  | ¢ |  |  | ¢ |  |
| Traffic Vol, veh/h | 1 | 1 | 0 | 10 | 0 | 30 | 0 | 39 | 34 | 37 | 39 | 1 |
| Future Vol, veh/h | 1 | 1 | 0 | 10 | 0 | 30 | 0 | 39 | 34 | 37 | 39 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 8 | 8 | 8 |  | 8 | 8 | 8 | 8 | 8 |
| Mumt Flow | 1 | 1 | 0 | 11 | 0 | 33 | 0 | 42 | 37 | 40 | 42 | 1 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB |  |  | WB |  |  |  | NB |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  |  | SB |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  |  | 1 |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  |  | EB |  | WB |  |  |
| Conflicting Lanes Left | 1 |  |  | 1 |  |  |  | 1 |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  |  | WB |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 1 |  |  |  | 1 |  | 1 |  |  |
| HCM Control Delay | 7.4 |  |  | 7.2 |  |  |  | 7.3 |  | 7.8 |  |  |
| HCM LOS | A |  |  | A |  |  |  | A |  | A |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Vol Left, \% | $0 \%$ | $50 \%$ | $25 \%$ | $48 \%$ |
| Vol Thru, \% | $53 \%$ | $50 \%$ | $0 \%$ | $51 \%$ |
| Vol Right, \% | $47 \%$ | $0 \%$ | $75 \%$ | $1 \%$ |
| Sign Control | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 73 | 2 | 40 | 77 |
| LT Vol | 0 | 1 | 10 | 37 |
| Through Vol | 39 | 1 | 0 | 39 |
| RT Vol | 34 | 0 | 30 | 1 |
| Lane Flow Rate | 79 | 2 | 43 | 84 |
| Geometry Grp | 1 | 1 | 1 | 1 |
| Degree of Util (X) | 0.086 | 0.003 | 0.047 | 0.099 |
| Departure Headway (Hd) | 3.9 | 4.35 | 3.918 | 4.265 |
| Convergence, Y/N | Yes | Yes | Yes | Yes |
| Cap | 915 | 812 | 902 | 839 |
| Service Time | 1.939 | 2.434 | 1.995 | 2.296 |
| HCM Lane V/C Ratio | 0.086 | 0.002 | 0.048 | 0.1 |
| HCM Control Delay | 7.3 | 7.4 | 7.2 | 7.8 |
| HCM Lane LOS | A | A | A | A |
| HCM 95th-tile Q | 0.3 | 0 | 0.1 | 0.3 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.7 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | -1 | F |  | 1 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 6 | 137 | 32 | 59 | 12 | 1 |
| Future Vol, veh/h | 6 | 137 | 32 | 59 | 12 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | Stop |
| Storage Length | - | - | - | - | 0 | 150 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 7 | 149 | 35 | 64 | 13 | 1 |


| Major/Minor M | Major1 |  | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 99 | 0 | - | 0 | 229 | 67 |  |
| Stage 1 | - | - | - - | - | 67 | - |  |
| Stage 2 | - | - | - - | - | 162 | - |  |
| Critical Hdwy | 4.18 | - | - - | - | 6.48 | 6.28 |  |
| Critical Hdwy Stg 1 | - | - | - - | - | 5.48 | - |  |
| Critical Hdwy Stg 2 | - | - | - - | - | 5.48 | - |  |
| Follow-up Hdwy | 2.272 | - | - - | - | 3.572 | 3.372 |  |
| Pot Cap-1 Maneuver | 1457 | - | - - | - | 746 | 980 |  |
| Stage 1 | - | - | - - | - | 941 | - |  |
| Stage 2 | - | - | - - | - | 853 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1457 | - | - - | - | 742 | 980 |  |
| Mov Cap-2 Maneuver | - | - | - - | - | 742 | - |  |
| Stage 1 | - | - | - - | - | 941 | - |  |
| Stage 2 | - | - | - - | - | 849 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 0.3 |  | 0 |  | 9.8 |  |  |
| HCM LOS |  |  |  |  | A |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 SBLn2 |  |  |  |
| Capacity (veh/h) |  | 1457 | , | - | - | 742 | 980 |
| HCM Lane V/C Ratio |  | 0.004 | - | - | - | 0.018 | 0.001 |
| HCM Control Delay (s) |  | 7.5 | 0 | - | - | 9.9 | 8.7 |
| HCM Lane LOS |  | A | A | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 0.1 | 0 |


| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh | 7.5 |
| Intersection LOS | A |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | ¢ |  |  | $\uparrow$ |  |  | ¢ |  |  | ¢ |  |
| Traffic Vol, veh/h | 1 | 1 | 0 | 10 | 0 | 30 | 0 | 39 | 34 | 37 | 39 | 1 |
| Future Vol, veh/h | 1 | 1 | 0 | 10 | 0 | 30 | 0 | 39 | 34 | 37 | 39 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 |
| Mumt Flow | 1 | 1 | 0 | 12 | 0 | 35 | 0 | 45 | 39 | 43 | 45 | 1 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB |  |  | WB |  |  |  | NB |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  |  | SB |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  |  | 1 |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  |  | EB |  | WB |  |  |
| Conflicting Lanes Left | 1 |  |  | 1 |  |  |  | 1 |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  |  | WB |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 1 |  |  |  | 1 |  | 1 |  |  |
| HCM Control Delay | 7.6 |  |  | 7.2 |  |  |  | 7.3 |  | 7.8 |  |  |
| HCM LOS | A |  |  | A |  |  |  | A |  | A |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Vol Left, \% | $0 \%$ | $50 \%$ | $25 \%$ | $48 \%$ |
| Vol Thru, \% | $53 \%$ | $50 \%$ | $0 \%$ | $51 \%$ |
| Vol Right, \% | $47 \%$ | $0 \%$ | $75 \%$ | $1 \%$ |
| Sign Control | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 73 | 2 | 40 | 77 |
| LT Vol | 0 | 1 | 10 | 37 |
| Through Vol | 39 | 1 | 0 | 39 |
| RT Vol | 34 | 0 | 30 | 1 |
| Lane Flow Rate | 84 | 2 | 46 | 89 |
| Geometry Grp | 1 | 1 | 1 | 1 |
| Degree of Util (X) | 0.091 | 0.003 | 0.05 | 0.105 |
| Departure Headway (Hd) | 3.908 | 4.472 | 3.935 | 4.273 |
| Convergence, Y/N | Yes | Yes | Yes | Yes |
| Cap | 913 | 789 | 897 | 837 |
| Service Time | 1.95 | 2.561 | 2.015 | 2.306 |
| HCM Lane V/C Ratio | 0.092 | 0.003 | 0.051 | 0.106 |
| HCM Control Delay | 7.3 | 7.6 | 7.2 | 7.8 |
| HCM Lane LOS | A | A | A | A |
| HCM 95th-tile Q | 0.3 | 0 | 0.2 | 0.4 |



Mullan 2021 AM Peak Hour Analysis \& TW Intersection
7: Mullan Rd \& Pulp Mill Rd


\section*{| NBL | NBT | NBREBLn1WBLn1 | SBL | SBT | SBR |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 1562 | - | -682 | 921 | 1513 | - | - |}


| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh $\quad 7.7$ |  |
| Intersection LOS | A |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | ¢ |  |  | $\uparrow$ |  |  | ¢ |  | ${ }^{7}$ | $\hat{\beta}$ |  |
| Traffic Vol, veh/h | 1 | 1 | 0 | 10 | 0 | 30 | 0 | 39 | 34 | 37 | 39 | 1 |
| Future Vol, veh/h | 1 | 1 | 0 | 10 | 0 | 30 | 0 | 39 | 34 | 37 | 39 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 |
| Mumt Flow | 1 | 1 | 0 | 12 | 0 | 35 | 0 | 45 | 39 | 43 | 45 | 1 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 0 |
| Approach | EB |  |  | WB |  |  |  | NB |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  |  | SB |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  |  | 2 |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  |  | EB |  | WB |  |  |
| Conflicting Lanes Left | 2 |  |  | 1 |  |  |  | 1 |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  |  | WB |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 2 |  |  |  | 1 |  | 1 |  |  |
| HCM Control Delay | 7.6 |  |  | 7.3 |  |  |  | 7.5 |  | 8.1 |  |  |
| HCM LOS | A |  |  | A |  |  |  | A |  | A |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 | SBLn2 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $0 \%$ | $50 \%$ | $25 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $53 \%$ | $50 \%$ | $0 \%$ | $0 \%$ | $97 \%$ |
| Vol Right, \% | $47 \%$ | $0 \%$ | $75 \%$ | $0 \%$ | $3 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 73 | 2 | 40 | 37 | 40 |
| LT Vol | 0 | 1 | 10 | 37 | 0 |
| Through Vol | 39 | 1 | 0 | 0 | 39 |
| RT Vol | 34 | 0 | 30 | 0 | 1 |
| Lane Flow Rate | 84 | 2 | 46 | 43 | 46 |
| Geometry Grp | 5 | 2 | 2 | 7 | 7 |
| Degree of Util (X) | 0.094 | 0.003 | 0.052 | 0.062 | 0.061 |
| Departure Headway (Hd) | 4.006 | 4.589 | 4.043 | 5.264 | 4.746 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 887 | 784 | 891 | 680 | 754 |
| Service Time | 2.066 | 2.591 | 2.043 | 3 | 2.481 |
| HCM Lane V/C Ratio | 0.095 | 0.003 | 0.052 | 0.063 | 0.061 |
| HCM Control Delay | 7.5 | 7.6 | 7.3 | 8.4 | 7.8 |
| HCM Lane LOS | A | A | A | A | A |
| HCM 95th-tile Q | 0.3 | 0 | 0.2 | 0.2 | 0.2 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.7 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | 1 | 4 | $\mathbf{F}$ |  | 1 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 6 | 137 | 32 | 59 | 12 | 1 |
| Future Vol, veh/h | 6 | 137 | 32 | 59 | 12 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | Stop |
| Storage Length | 150 | - | - | - | 0 | 150 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 7 | 158 | 37 | 68 | 14 | 1 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 105 | 0 | 0 | 0 | 243 | 71 |  |
| Stage 1 | - |  | - | - | 71 | - |  |
| Stage 2 | - |  | - | - | 172 | - |  |
| Critical Hdwy | 4.18 |  | - | - | 6.48 | 6.28 |  |
| Critical Hdwy Stg 1 | - |  | - | - | 5.48 | - |  |
| Critical Hdwy Stg 2 | - |  | - | - | 5.48 | - |  |
| Follow-up Hdwy | 2.272 |  | - | - | 3.572 | 3.372 |  |
| Pot Cap-1 Maneuver | 1450 |  | - | - | 732 | 975 |  |
| Stage 1 | - |  | - | - | 937 | - |  |
| Stage 2 | - |  | - | - | 844 | - |  |
| Platoon blocked, \% |  |  | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1450 |  | - | - | 728 | 975 |  |
| Mov Cap-2 Maneuver | - |  | - | - | 728 | - |  |
| Stage 1 | - |  | - - | - | 937 | - |  |
| Stage 2 | - | - | - - | - | 840 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 0.3 |  | 0 |  | 9.9 |  |  |
| HCM LOS |  |  |  |  | A |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| Capacity (veh/h) |  | 1450 | - | - | Wr | 728 | 975 |
| HCM Lane V/C Ratio |  | 0.005 | 5 | - | - | 0.019 | 0.001 |
| HCM Control Delay (s) |  | 7.5 | , | - | - | 10 | 8.7 |
| HCM Lane LOS |  | A | A | - | - | B | A |
| HCM 95th \%tile Q(veh) |  | 0 | 0 | - | - | 0.1 | 0 |




| Intersection |  |
| :--- | ---: | :--- |
| Intersection Delay, s/veh | 7.8 |
| Intersection LOS | A |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | * |  |  | * |  |  | \& |  |  | \$ |  |
| Traffic Vol, veh/h | 1 | 1 | 0 | 10 | 0 | 30 | 0 | 39 | 34 | 37 | 39 | 1 |
| Future Vol, veh/h | 1 | 1 | 0 | 10 | 0 | 30 | 0 | 39 | 34 | 37 | 39 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 2 | 2 | 0 | 16 | 0 | 47 | 0 | 61 | 53 | 58 | 61 | 2 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB |  |  | WB |  |  |  | NB |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  |  | SB |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  |  | 1 |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  |  | EB |  | WB |  |  |
| Conflicting Lanes Left | 1 |  |  | 1 |  |  |  | 1 |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  |  | WB |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 1 |  |  |  | 1 |  | 1 |  |  |
| HCM Control Delay | 7.7 |  |  | 7.5 |  |  |  | 7.6 |  | 8.1 |  |  |
| HCM LOS | A |  |  | A |  |  |  | A |  | A |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Vol Left, \% | $0 \%$ | $50 \%$ | $25 \%$ | $48 \%$ |
| Vol Thru, \% | $53 \%$ | $50 \%$ | $0 \%$ | $51 \%$ |
| Vol Right, \% | $47 \%$ | $0 \%$ | $75 \%$ | $1 \%$ |
| Sign Control | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 73 | 2 | 40 | 77 |
| LT Vol | 0 | 1 | 10 | 37 |
| Through Vol | 39 | 1 | 0 | 39 |
| RT Vol | 34 | 0 | 30 | 1 |
| Lane Flow Rate | 113 | 3 | 62 | 120 |
| Geometry Grp | 1 | 1 | 1 | 1 |
| Degree of Util (X) | 0.125 | 0.004 | 0.072 | 0.144 |
| Departure Headway (Hd) | 3.961 | 4.72 | 4.153 | 4.325 |
| Convergence, Y/N | Yes | Yes | Yes | Yes |
| Cap | 895 | 762 | 868 | 823 |
| Service Time | 2.03 | 2.722 | 2.153 | 2.382 |
| HCM Lane V/C Ratio | 0.126 | 0.004 | 0.071 | 0.146 |
| HCM Control Delay | 7.6 | 7.7 | 7.5 | 8.1 |
| HCM Lane LOS | A | A | A | A |
| HCM 95th-tile Q | 0.4 | 0 | 0.2 | 0.5 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.7 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{\uparrow}$ | $\mathbf{F}$ |  | 1 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 6 | 137 | 32 | 59 | 12 | 1 |
| Future Vol, veh/h | 6 | 137 | 32 | 59 | 12 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | Stop |
| Storage Length | - | - | - | - | 0 | 150 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 9 | 213 | 50 | 92 | 19 | 2 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 141 | 0 | - | 0 | 328 | 96 |  |
| Stage 1 | - | - | - - | - | 96 | - |  |
| Stage 2 | - | - | - - | - | 232 | - |  |
| Critical Hdwy | 4.18 | - | - - | - | 6.48 | 6.28 |  |
| Critical Hdwy Stg 1 | - | - | - - | - | 5.48 | - |  |
| Critical Hdwy Stg 2 | - | - | - - | - | 5.48 | - |  |
| Follow-up Hdwy | 2.272 | - | - - | - | 3.572 | 3.372 |  |
| Pot Cap-1 Maneuver | 1406 | - | - - | - | 654 | 944 |  |
| Stage 1 | - | - | - - | - | 913 | - |  |
| Stage 2 | - | - | - - | - | 793 | - |  |
| Platoon blocked, \% |  | - | - - | - |  |  |  |
| Mov Cap-1 Maneuver | 1406 | - | - - | - | 649 | 944 |  |
| Mov Cap-2 Maneuver | - | - | - - | - | 649 | - |  |
| Stage 1 | - | - | - - | - | 913 | - |  |
| Stage 2 | - | - | - - | - | 787 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 0.3 |  | 0 |  | 10.6 |  |  |
| HCM LOS |  |  |  |  | B |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 SBLn2 |  |  |  |
| Capacity (veh/h) |  | 1406 |  | - | - | 649 | 944 |
| HCM Lane V/C Ratio |  | 0.007 | , | - | - | 0.029 | 0.002 |
| HCM Control Delay (s) |  | 7.6 | 0 | - | - | 10.7 | 8.8 |
| HCM Lane LOS |  | A | A | - | - | B | A |
| HCM 95th \%tile Q(veh) |  | 0 | 0 | - | - | 0.1 | 0 |



| Intersection |  |
| :--- | ---: | :--- |
| Intersection Delay, s/veh | 7.9 |
| Intersection LOS | A |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | ¢ |  |  | $\uparrow$ |  |  | $\uparrow$ |  | ${ }^{7}$ | $\hat{\beta}$ |  |
| Traffic Vol, veh/h | 1 | 1 | 0 | 10 | 0 | 30 | 0 | 39 | 34 | 37 | 39 | 1 |
| Future Vol, veh/h | 1 | 1 | 0 | 10 | 0 | 30 | 0 | 39 | 34 | 37 | 39 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 2 | 2 | 0 | 16 | 0 | 47 | 0 | 61 | 53 | 58 | 61 | 2 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 0 |
| Approach | EB |  |  | WB |  |  |  | NB |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  |  | SB |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  |  | 2 |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  |  | EB |  | WB |  |  |
| Conflicting Lanes Left | 2 |  |  | 1 |  |  |  | 1 |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  |  | WB |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 2 |  |  |  | 1 |  | 1 |  |  |
| HCM Control Delay | 7.8 |  |  | 7.5 |  |  |  | 7.8 |  | 8.3 |  |  |
| HCM LOS | A |  |  | A |  |  |  | A |  | A |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 | SBLn2 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $0 \%$ | $50 \%$ | $25 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $53 \%$ | $50 \%$ | $0 \%$ | $0 \%$ | $97 \%$ |
| Vol Right, \% | $47 \%$ | $0 \%$ | $75 \%$ | $0 \%$ | $3 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 73 | 2 | 40 | 37 | 40 |
| LT Vol | 0 | 1 | 10 | 37 | 0 |
| Through Vol | 39 | 1 | 0 | 0 | 39 |
| RT Vol | 34 | 0 | 30 | 0 | 1 |
| Lane Flow Rate | 113 | 3 | 62 | 58 | 62 |
| Geometry Grp | 5 | 2 | 2 | 7 | 7 |
| Degree of Util (X) | 0.128 | 0.004 | 0.072 | 0.085 | 0.083 |
| Departure Headway (Hd) | 4.058 | 4.752 | 4.185 | 5.31 | 4.792 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 870 | 757 | 861 | 672 | 744 |
| Service Time | 2.144 | 2.754 | 2.185 | 3.063 | 2.544 |
| HCM Lane V/C Ratio | 0.13 | 0.004 | 0.072 | 0.086 | 0.083 |
| HCM Control Delay | 7.8 | 7.8 | 7.5 | 8.6 | 8 |
| HCM Lane LOS | A | A | A | A | A |
| HCM 95th-tile Q | 0.4 | 0 | 0.2 | 0.3 | 0.3 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.7 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | 1 | $\mathbf{4}$ | $\mathbf{F}$ |  | l | $\mathbf{7}$ |
| Traffic Vol, veh/h | 6 | 137 | 32 | 59 | 12 | 1 |
| Future Vol, veh/h | 6 | 137 | 32 | 59 | 12 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | Stop |
| Storage Length | 150 | - | - | - | 0 | 150 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 9 | 213 | 50 | 92 | 19 | 2 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 141 | 0 | - | 0 | 328 | 96 |  |
| Stage 1 | - | - | - - | - | 96 | - |  |
| Stage 2 | - | - | - - | - | 232 | - |  |
| Critical Hdwy | 4.18 | - | - - | - | 6.48 | 6.28 |  |
| Critical Hdwy Stg 1 | - | - | - - | - | 5.48 | - |  |
| Critical Hdwy Stg 2 | - | - | - - | - | 5.48 | - |  |
| Follow-up Hdwy | 2.272 | - | - - | - | 3.572 | 3.372 |  |
| Pot Cap-1 Maneuver | 1406 | - | - - | - | 654 | 944 |  |
| Stage 1 | - | - | - - | - | 913 | - |  |
| Stage 2 | - | - | - - | - | 793 | - |  |
| Platoon blocked, \% |  | - | - - | - |  |  |  |
| Mov Cap-1 Maneuver | 1406 | - | - - | - | 650 | 944 |  |
| Mov Cap-2 Maneuver | - | - | - - | - | 650 | - |  |
| Stage 1 | - | - | - - | - | 913 | - |  |
| Stage 2 | - | - | - - | - | 788 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 0.3 |  | 0 |  | 10.6 |  |  |
| HCM LOS |  |  |  |  | B |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| Capacity (veh/h) |  | 1406 | - | - | - | 650 | 944 |
| HCM Lane V/C Ratio |  | 0.007 | - | - | - | 0.029 | 0.002 |
| HCM Control Delay (s) |  | 7.6 | - | - | - | 10.7 | 8.8 |
| HCM Lane LOS |  | A | A | - | - | B | A |
| HCM 95th \%tile Q(veh) |  | 0 | A | - | - | 0.1 | 0 |




| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh $\quad 7.7$ |  |
| Intersection LOS | A |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | ¢ |  |  | ¢ |  |  | \$ |  |  | \$ |  |
| Traffic Vol, veh/h | 1 | 1 | 0 | 37 | 1 | 52 | 1 | 66 | 12 | 20 | 45 | 1 |
| Future Vol, veh/h | 1 | 1 | 0 | 37 | 1 | 52 | 1 | 66 | 12 | 20 | 45 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 |
| Mumt Flow | 1 | 1 | 0 | 40 | 1 | 57 | 1 | 72 | 13 | 22 | 49 | 1 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 1 |  |  | 1 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 1 |  |  | 1 |  |  | 1 |  |  |
| HCM Control Delay | 7.6 |  |  | 7.6 |  |  | 7.7 |  |  | 7.8 |  |  |
| HCM LOS | A |  |  | A |  |  | A |  |  | A |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Vol Left, \% | $1 \%$ | $50 \%$ | $41 \%$ | $30 \%$ |
| Vol Thru, \% | $84 \%$ | $50 \%$ | $1 \%$ | $68 \%$ |
| Vol Right, \% | $15 \%$ | $0 \%$ | $58 \%$ | $2 \%$ |
| Sign Control | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 79 | 2 | 90 | 66 |
| LT Vol | 1 | 1 | 37 | 20 |
| Through Vol | 66 | 1 | 1 | 45 |
| RT Vol | 12 | 0 | 52 | 1 |
| Lane Flow Rate | 86 | 2 | 98 | 72 |
| Geometry Grp | 1 | 1 | 1 | 1 |
| Degree of Util (X) | 0.1 | 0.003 | 0.11 | 0.086 |
| Departure Headway (Hd) | 4.178 | 4.597 | 4.044 | 4.328 |
| Convergence, Y/N | Yes | Yes | Yes | Yes |
| Cap | 849 | 783 | 873 | 820 |
| Service Time | 2.245 | 2.597 | 2.133 | 2.398 |
| HCM Lane V/C Ratio | 0.101 | 0.003 | 0.112 | 0.088 |
| HCM Control Delay | 7.7 | 7.6 | 7.6 | 7.8 |
| HCM Lane LOS | A | A | A | A |
| HCM 95th-tile Q | 0.3 | 0 | 0.4 | 0.3 |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 2.4 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | $\uparrow$ |  | ${ }^{*}$ | 「 |
| Traffic Vol, veh/h | 1 | 85 | 136 | 24 | 65 | 6 |
| Future Vol, veh/h | 1 | 85 | 136 | 24 | 65 | 6 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | Stop |
| Storage Length | - | - | - | - | 0 | 150 |
| Veh in Median Storage, \# | \# | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 1 | 92 | 148 | 26 | 71 | 7 |



| Intersection |  |
| :--- | ---: | :--- |
| Intersection Delay, s/veh | 7.8 |
| Intersection LOS | A |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | * |  |  | * |  |  | \& |  |  | * |  |
| Traffic Vol, veh/h | 1 | 1 | 0 | 37 | 1 | 52 | 1 | 66 | 12 | 20 | 45 | 1 |
| Future Vol, veh/h | 1 | 1 | 0 | 37 | 1 | 52 | 1 | 66 | 12 | 20 | 45 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 1 | 1 | 0 | 43 | 1 | 60 | 1 | 76 | 14 | 23 | 52 | 1 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 1 |  |  | 1 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 1 |  |  | 1 |  |  | 1 |  |  |
| HCM Control Delay | 7.6 |  |  | 7.7 |  |  | 7.8 |  |  | 7.9 |  |  |
| HCM LOS | A |  |  | A |  |  | A |  |  | A |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Vol Left, \% | $1 \%$ | $50 \%$ | $41 \%$ | $30 \%$ |
| Vol Thru, \% | $84 \%$ | $50 \%$ | $1 \%$ | $68 \%$ |
| Vol Right, \% | $15 \%$ | $0 \%$ | $58 \%$ | $2 \%$ |
| Sign Control | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 79 | 2 | 90 | 66 |
| LT Vol | 1 | 1 | 37 | 20 |
| Through Vol | 66 | 1 | 1 | 45 |
| RT Vol | 12 | 0 | 52 | 1 |
| Lane Flow Rate | 91 | 2 | 104 | 76 |
| Geometry Grp | 1 | 1 | 1 | 1 |
| Degree of Util (X) | 0.106 | 0.003 | 0.117 | 0.092 |
| Departure Headway (Hd) | 4.191 | 4.627 | 4.061 | 4.343 |
| Convergence, Y/N | Yes | Yes | Yes | Yes |
| Cap | 846 | 778 | 868 | 816 |
| Service Time | 2.264 | 2.627 | 2.155 | 2.417 |
| HCM Lane V/C Ratio | 0.108 | 0.003 | 0.12 | 0.093 |
| HCM Control Delay | 7.8 | 7.6 | 7.7 | 7.9 |
| HCM Lane LOS | A | A | A | A |
| HCM 95th-tile Q | 0.4 | 0 | 0.4 | 0.3 |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 2.4 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | $\uparrow$ |  | ${ }^{*}$ | 「 |
| Traffic Vol, veh/h | 1 | 85 | 136 | 24 | 65 | 6 |
| Future Vol, veh/h | 1 | 85 | 136 | 24 | 65 | 6 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | Stop |
| Storage Length | - | - | - | - | 0 | 150 |
| Veh in Median Storage, \# | \# | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 1 | 98 | 157 | 28 | 75 | 7 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 184 | 0 | - | 0 | 271 | 171 |  |
| Stage 1 | - | - | - - | - | 171 | - |  |
| Stage 2 | - | - | - - | - | 100 | - |  |
| Critical Hdwy | 4.18 | - | - - | - | 6.48 | 6.28 |  |
| Critical Hdwy Stg 1 | - | - | - - | - | 5.48 | - |  |
| Critical Hdwy Stg 2 | - | - | - - | - | 5.48 | - |  |
| Follow-up Hdwy | 2.272 | - | - - | - | 3.572 | 3.372 |  |
| Pot Cap-1 Maneuver | 1355 | - | - - | - | 706 | 857 |  |
| Stage 1 | - | - | - - | - | 845 | - |  |
| Stage 2 | - | - | - - | - | 909 | - |  |
| Platoon blocked, \% |  | - | - - | - |  |  |  |
| Mov Cap-1 Maneuver | 1355 | - | - - | - | 705 | 857 |  |
| Mov Cap-2 Maneuver | - | - | - - | - | 705 | - |  |
| Stage 1 | - | - | - - | - | 845 | - |  |
| Stage 2 | - | - | - - | - | 908 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 0.1 |  | 0 |  | 10.6 |  |  |
| HCM LOS |  |  |  |  | B |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 SBLn2 |  |  |  |
| Capacity (veh/h) |  | 1355 |  | - | - | 705 | 857 |
| HCM Lane V/C Ratio |  | 0.001 | - | - | - | 0.106 | 0.008 |
| HCM Control Delay (s) |  | 7.7 | 0 | - | - | 10.7 | 9.2 |
| HCM Lane LOS |  | A | A | - | - | B | A |
| HCM 95th \%tile Q(veh) |  | 0 | 0 | - | - | 0.4 | 0 |



| Intersection |  |
| :--- | ---: | :--- |
| Intersection Delay, s/veh | 7.9 |
| Intersection LOS | A |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | ¢ |  |  | $\uparrow$ |  |  | \$ |  | ${ }^{7}$ | $\hat{\dagger}$ |  |
| Traffic Vol, veh/h | 1 | 1 | 0 | 37 | 1 | 52 | 1 | 66 | 12 | 20 | 45 | 1 |
| Future Vol, veh/h | 1 | 1 | 0 | 37 | 1 | 52 | 1 | 66 | 12 | 20 | 45 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 |
| Mumt Flow | 1 | 1 | 0 | 43 | 1 | 60 | 1 | 76 | 14 | 23 | 52 | 1 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 0 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  | 2 |  |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 2 |  |  | 1 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 2 |  |  | 1 |  |  | 1 |  |  |
| HCM Control Delay | 7.7 |  |  | 7.7 |  |  | 7.9 |  |  | 8.1 |  |  |
| HCM LOS | A |  |  | A |  |  | A |  |  | A |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 | SBLn2 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $1 \%$ | $50 \%$ | $41 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $84 \%$ | $50 \%$ | $1 \%$ | $0 \%$ | $98 \%$ |
| Vol Right, \% | $15 \%$ | $0 \%$ | $58 \%$ | $0 \%$ | $2 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 79 | 2 | 90 | 20 | 46 |
| LT Vol | 1 | 1 | 37 | 20 | 0 |
| Through Vol | 66 | 1 | 1 | 0 | 45 |
| RT Vol | 12 | 0 | 52 | 0 | 1 |
| Lane Flow Rate | 91 | 2 | 104 | 23 | 53 |
| Geometry Grp | 5 | 2 | 2 | 7 | 7 |
| Degree of Util (X) | 0.108 | 0.003 | 0.12 | 0.034 | 0.071 |
| Departure Headway (Hd) | 4.29 | 4.647 | 4.174 | 5.372 | 4.855 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 823 | 774 | 864 | 661 | 731 |
| Service Time | 2.38 | 2.65 | 2.174 | 3.148 | 2.631 |
| HCM Lane V/C Ratio | 0.111 | 0.003 | 0.12 | 0.035 | 0.073 |
| HCM Control Delay | 7.9 | 7.7 | 7.7 | 8.3 | 8 |
| HCM Lane LOS | A | A | A | A | A |
| HCM 95th-tile Q | 0.4 | 0 | 0.4 | 0.1 | 0.2 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.4 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | i | 4 | $\mathbf{F}$ |  | 1 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 1 | 85 | 136 | 24 | 65 | 6 |
| Future Vol, veh/h | 1 | 85 | 136 | 24 | 65 | 6 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | Stop |
| Storage Length | 150 | - | - | - | 0 | 150 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 1 | 98 | 157 | 28 | 75 | 7 |





| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh | 8.2 |
| Intersection LOS | A |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | $\uparrow$ |  |  | * |  |  | ¢ |  |  | ¢ |  |
| Traffic Vol, veh/h | 1 | 1 | 0 | 37 | 1 | 52 | 1 | 66 | 12 | 20 | 45 | 1 |
| Future Vol, veh/h | 1 | 1 | 0 | 37 | 1 | 52 | 1 | 66 | 12 | 20 | 45 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 2 | 2 | 0 | 58 | 2 | 81 | 2 | 103 | 19 | 31 | 70 | 2 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 1 |  |  | 1 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 1 |  |  | 1 |  |  | 1 |  |  |
| HCM Control Delay | 7.9 |  |  | 8.2 |  |  | 8.2 |  |  | 8.2 |  |  |
| HCM LOS | A |  |  | A |  |  | A |  |  | A |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Vol Left, \% | $1 \%$ | $50 \%$ | $41 \%$ | $30 \%$ |
| Vol Thư, \% | $84 \%$ | $50 \%$ | $1 \%$ | $68 \%$ |
| Vol Right, \% | $15 \%$ | $0 \%$ | $58 \%$ | $2 \%$ |
| Sign Control | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 79 | 2 | 90 | 66 |
| LT Vol | 1 | 1 | 37 | 20 |
| Through Vol | 66 | 1 | 1 | 45 |
| RT Vol | 12 | 0 | 52 | 1 |
| Lane Flow Rate | 123 | 3 | 140 | 103 |
| Geometry Grp | 1 | 1 | 1 | 1 |
| Degree of Util (X) | 0.15 | 0.004 | 0.167 | 0.13 |
| Departure Headway (Hd) | 4.387 | 4.815 | 4.295 | 4.545 |
| Convergence, Y/N | Yes | Yes | Yes | Yes |
| Cap | 820 | 745 | 838 | 791 |
| Service Time | 2.4 | 2.835 | 2.31 | 2.559 |
| HCM Lane V/C Ratio | 0.15 | 0.004 | 0.167 | 0.13 |
| HCM Control Delay | 8.2 | 7.9 | 8.2 | 8.2 |
| HCM Lane LOS | A | A | A | A |
| HCM 95th-tile Q | 0.5 | 0 | 0.6 | 0.4 |






| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh | 8.3 |
| Intersection LOS | A |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | $\uparrow$ |  |  | * |  |  | ¢ |  | ${ }^{7}$ | $\hat{\beta}$ |  |
| Traffic Vol, veh/h | 1 | 1 | 0 | 37 | 1 | 52 | 1 | 66 | 12 | 20 | 45 | 1 |
| Future Vol, veh/h | 1 | 1 | 0 | 37 | 1 | 52 | 1 | 66 | 12 | 20 | 45 | 1 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 2 | 2 | 0 | 58 | 2 | 81 | 2 | 103 | 19 | 31 | 70 | 2 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 0 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  | 2 |  |  | 1 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 2 |  |  | 1 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 1 |  |  | 2 |  |  | 1 |  |  | 1 |  |  |
| HCM Control Delay | 7.9 |  |  | 8.2 |  |  | 8.3 |  |  | 8.4 |  |  |
| HCM LOS | A |  |  | A |  |  | A |  |  | A |  |  |


| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 | SBLn2 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $1 \%$ | $50 \%$ | $41 \%$ | $100 \%$ | $0 \%$ |
| Vol Thru, \% | $84 \%$ | $50 \%$ | $1 \%$ | $0 \%$ | $98 \%$ |
| Vol Right, \% | $15 \%$ | $0 \%$ | $58 \%$ | $0 \%$ | $2 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 79 | 2 | 90 | 20 | 46 |
| LT Vol | 1 | 1 | 37 | 20 | 0 |
| Through Vol | 66 | 1 | 1 | 0 | 45 |
| RT Vol | 12 | 0 | 52 | 0 | 1 |
| Lane Flow Rate | 123 | 3 | 140 | 31 | 71 |
| Geometry Grp | 5 | 2 | 2 | 7 | 7 |
| Degree of Util (X) | 0.153 | 0.004 | 0.168 | 0.048 | 0.1 |
| Departure Headway (Hd) | 4.499 | 4.842 | 4.322 | 5.562 | 5.044 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes |
| Cap | 799 | 740 | 832 | 646 | 712 |
| Service Time | 2.516 | 2.863 | 2.334 | 3.279 | 2.761 |
| HCM Lane V/C Ratio | 0.154 | 0.004 | 0.168 | 0.048 | 0.1 |
| HCM Control Delay | 8.3 | 7.9 | 8.2 | 8.6 | 8.3 |
| HCM Lane LOS | A | A | A | A | A |
| HCM 95th-tile Q | 0.5 | 0 | 0.6 | 0.2 | 0.3 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.6 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | i | 4 | $\mathbf{F}$ |  | 1 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 1 | 85 | 136 | 24 | 65 | 6 |
| Future Vol, veh/h | 1 | 85 | 136 | 24 | 65 | 6 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | Stop |
| Storage Length | 150 | - | - | - | 0 | 150 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 8 | 8 | 8 | 8 | 8 | 8 |
| Mvmt Flow | 2 | 132 | 211 | 37 | 101 | 9 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 249 | 0 | 0 | 0 | 365 | 230 |  |
| Stage 1 | - |  | - | - | 230 | - |  |
| Stage 2 | - |  | - | - | 135 | - |  |
| Critical Hdwy | 4.18 |  | - | - | 6.48 | 6.28 |  |
| Critical Hdwy Stg 1 | - |  | - | - | 5.48 | - |  |
| Critical Hdwy Stg 2 | - |  | - | - | 5.48 | - |  |
| Follow-up Hdwy | 2.272 |  | - | - | 3.572 | 3.372 |  |
| Pot Cap-1 Maneuver | 1282 |  | - | - | 623 | 795 |  |
| Stage 1 | - |  | - | - | 794 | - |  |
| Stage 2 | - |  | - | - | 877 | - |  |
| Platoon blocked, \% |  |  | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1282 |  | - | - | 622 | 795 |  |
| Mov Cap-2 Maneuver | - |  | - - | - | 622 | - |  |
| Stage 1 | - |  | - - | - | 794 | - |  |
| Stage 2 | - | - | - - | - | 876 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 0.1 |  | 0 |  | 11.7 |  |  |
| HCM LOS |  |  |  |  | B |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| Capacity (veh/h) |  | 1282 | - |  | Wr | 622 | 795 |
| HCM Lane V/C Ratio |  | 0.001 | 1 | - | - | 0.162 | 0.012 |
| HCM Control Delay (s) |  | 7.8 | 8 | - | - | 11.9 | 9.6 |
| HCM Lane LOS |  | A | A | - | - | B | A |
| HCM 95th \%tile Q(veh) |  | 0 | 0 | - | - | 0.6 | 0 |




